RESOLUTION NO. 2017- 79

A RESOLUTION BY THE BOARD OF COUNTY COMMISSIONERS OF ST. JOHNS COUNTY, FLORIDA, APPROVING THE TERMS AND AUTHORIZING THE COUNTY ADMINISTRATOR, OR DESIGNEE, TO EXECUTE AMENDMENT NO. 7 TO THE TOWER ATTACHMENT COMMUNICATION SITE AGREEMENT WITH SPRINTCOM, INC., AS SUCCESSOR IN INTEREST TO NEXTEL SOUTH CORP. FOR MODIFICATIONS TO THEIR EQUIPMENT ON THE PONTE VEDRA ANNEX TOWER.

RECITALS

WHEREAS, St. Johns County and SprintCom, Inc., as successor in interest to Nextel South Corp. entered into a Tower Attachment Communication Site Agreement ("Agreement"), dated October 21, 1998, and approved in Resolution 98-159, on the Ponte Vedra Annex tower on Palm Valley Road; and,

WHEREAS, SprintCom will be modifying their equipment on the tower to enhance their service in the area.

NOW THEREFORE BE IT RESOLVED by the Board of County Commissioners of St. Johns County, Florida, as follows:

- **Section 1.** The above Recitals are incorporated by reference into the body of the Resolution and such Recitals are adopted as findings of fact.
- **Section2.** The Board of County Commissioners hereby approves and authorizes the County Administrator, or designee, to execute two originals of Amendment No. 7 the Agreement, attached hereto as Exhibit "A", incorporated by reference and made a part hereof, in substantially the form attached hereto, and any future amendments to the Agreement that do not substantially change the material terms and conditions of the Agreement.
- **Section 3.** To the extent that there are typographical errors that do not change the tone, tenor, or concept of this Resolution, then this Resolution may be revised without subsequent approval by the Board of County Commissioners.
- **Section 4.** The Clerk is instructed to record the Amendment No. 7 to the Agreement in the Public Records of St. Johns County, Florida.

PASSED AND ADOPTED by the	Board of County Commissioners of St. Johns County,
Florida, this 74 day of work	
	BOARD OF COUNTY COMMISSIONERS
	OF ST. JOHNS COUNTY, FLORIDA
	Ву:
	James K. Johns, Chair
ATTEST, Hunter S. Conrad, Clerk By: Jam Halterman	
Deputy Clerk	26/15

RENDITION DATE 3/9/17

(JA73XC004)

AMENDMENT NO. 7 TO TOWER ATTACHMENT COMMUNICATIONS SITE AGREEMENT

This Amendment No. 7 to Tower Attachment Communications Site Agreement (this "Amendment"), effective as of the date last signed below ("Effective Date"), amends a certain Tower Attachment Communications Site Agreement between SprintCom, Inc., a Kansas corporation, as successor in interest to Nextel South Corp., a Georgia corporation, ("Lessee"), and St. Johns County, a political subdivision of the State of Florida, through its Board of County Commissioners, ("Lessor"), dated October 21, 1998 (the "Lease"), as amended by First Amendment to Tower Attachment Communications Site Agreement dated March 18, 2002, and amended by Second Amendment to Tower Attachment Communications Site Agreement dated May 5, 2006, and amended by Amendment No. 3 to Tower Attachment Communications Site Agreement dated November 27, 2006, and amended by Amendment No. 4 to Tower Attachment Communications Site Agreement dated March 8, 2007, and amended by Amendment No. 5 to Tower Attachment Communications Site Agreement dated May 15, 2013, and further amended by Amendment No. 6 to Tower Attachment Communications Site Agreement dated August 22, 2015 (collectively, the "Agreement").

BACKGROUND

WHEREAS, Lessee desires to modify its installation on the Premises by adding or swapping out antennas and other equipment on the Tower, as more particularly described in Exhibit C-5 annexed hereto, and Lessee and Lessor desire to modify the provisions of the Agreement as provided below.

AGREEMENT

For good and valuable consideration the receipt and sufficiency of which are acknowledged, Lessor and Lessee agree as follows:

1. Modification to the Premises. Exhibit C-2, Exhibit C-3 and Exhibit C-4 to the Agreement are hereby amended to include the modifications identified on Exhibit C-5 (the "Modifications"), attached hereto and made a part hereof. Exhibit C-5 supplements Exhibit C-2, Exhibit C-3 and Exhibit C-4 to the Agreement, and shall not be deemed to supersede or otherwise modify Exhibit C-2, Exhibit C-3 or Exhibit C-4 or any part thereof except to the extent specifically set forth in Exhibit C-5. Upon full execution of this Amendment, Lessee is permitted to do all work necessary to prepare, maintain and alter the Site to install or otherwise modify the Premises, all as more fully described and contemplated in Exhibit C-5.

2. General Terms and Conditions.

- a. All capitalized terms used in this Amendment, unless otherwise defined herein, will have the same meaning as the terms contained in the Agreement.
- b. In case of any inconsistencies between the terms and conditions contained in the Agreement and the terms and conditions contained in this Amendment, the terms and conditions herein will control. Except as set forth below, all provisions of the Agreement are ratified and remain unchanged and in full force and effect.
- c. This Amendment may be executed in duplicate counterparts, each of which will be deemed an original.
- d. Each of the parties represents and warrants that it has the right, power, legal capacity and authority to enter into and perform its respective obligations under this Amendment.

SIGNATURES ON FOLLOWING PAGE

The parties have executed this Amendment as of the Effective Date.

	Lessee
Signed, sealed and delivered in the presence of:	SprintCom, Inc. a Kansas corporation
	By:
Printed Name:	Printed Name:
	Title:
Printed Name:	Date:
	(Date must be completed)
	Lessor
Signed, sealed and delivered in the presence of:	St. Johns County, a political subdivision of the State of Florida, through its Board of County Commissioners
	By:
Printed Name:	Printed Name:
	Title:
Printed Name:	
	(Date must be completed)

Exhibit C-4

Schedule of Equipment

Existing Equipment

- 3 RFS APXVERR18-C Antennas
- 3 H+S TSZ 999 066/xxxM 1 1/4" lines
- 3 Ericsson 800 ESMR Filters
- 9 Ericsson ACU-A20-N RETS
- 3 Ericsson RRUS 11 @ 800 MHz
- 3 Ericsson RRU 31 @ 1900 MhZ

Equipment to be Removed:

N/A

Equipment to be Added

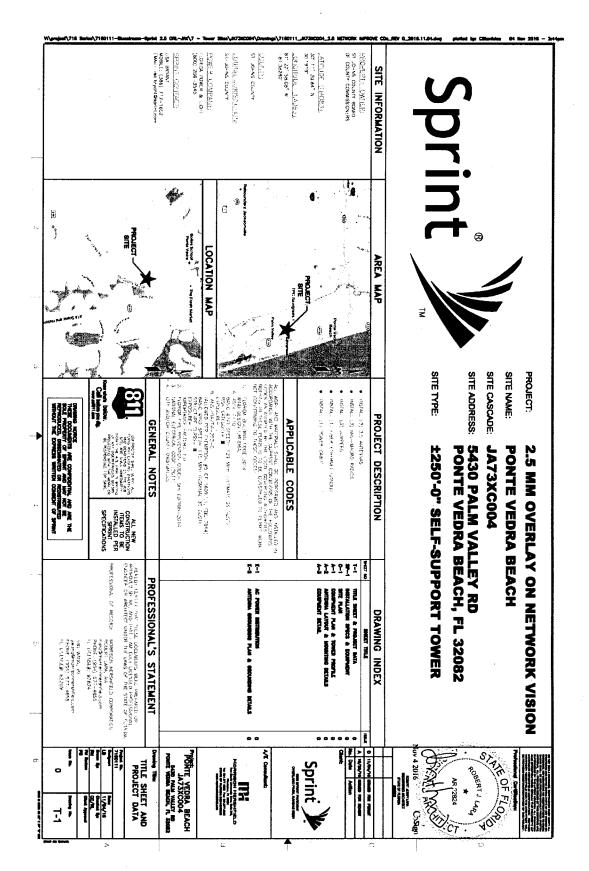
- 3 Alpha Wireless AW3286 Panel Antenna
- 3 Nokia FWHR TMA

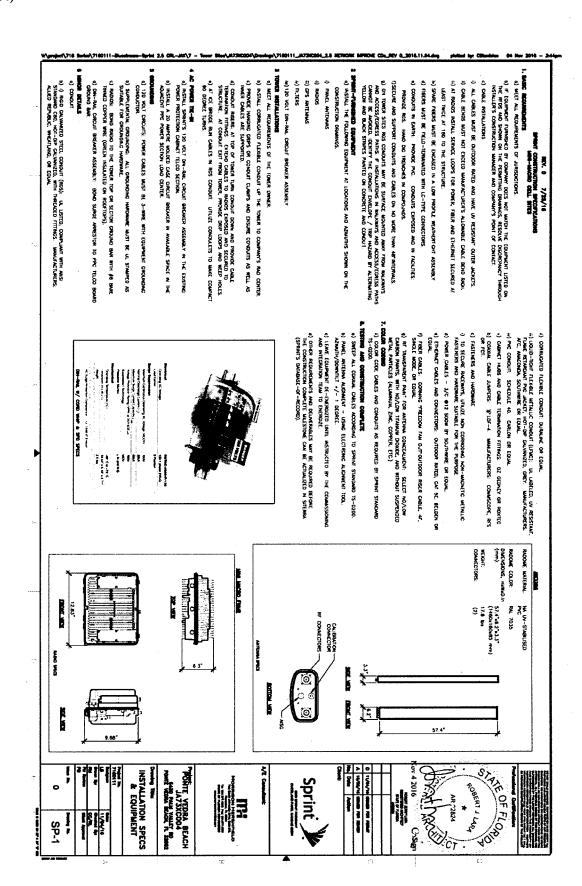
Conduit with 3 Fiber and 3 Cat5e

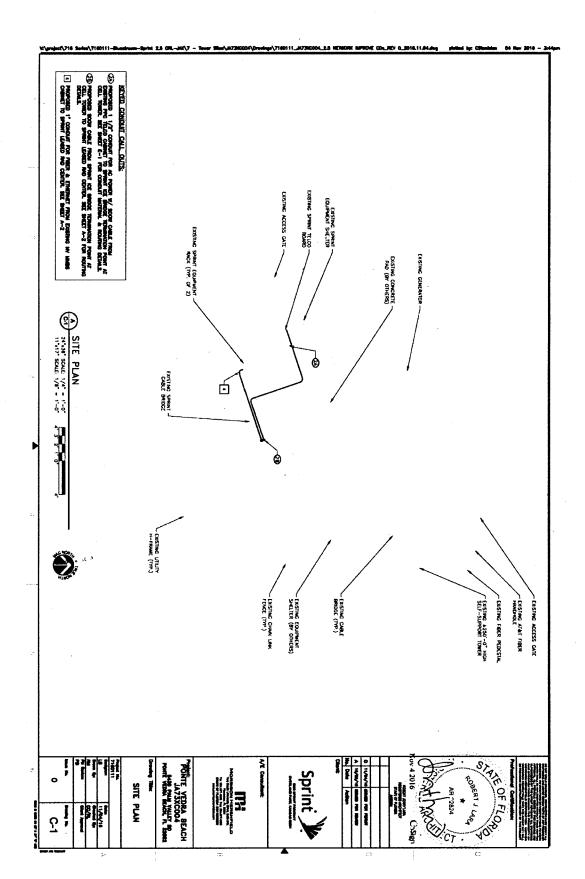
1 SOOW power cable

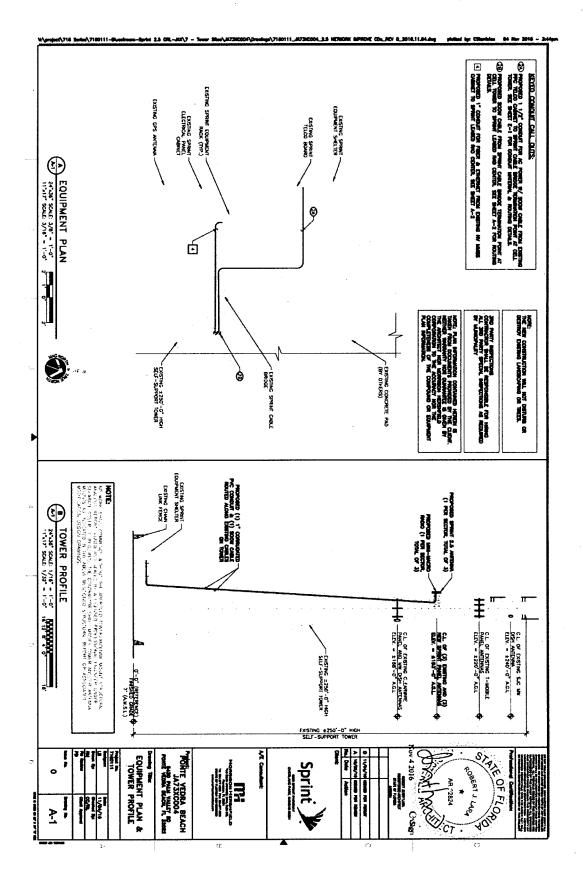
Final Equipment Configuration:

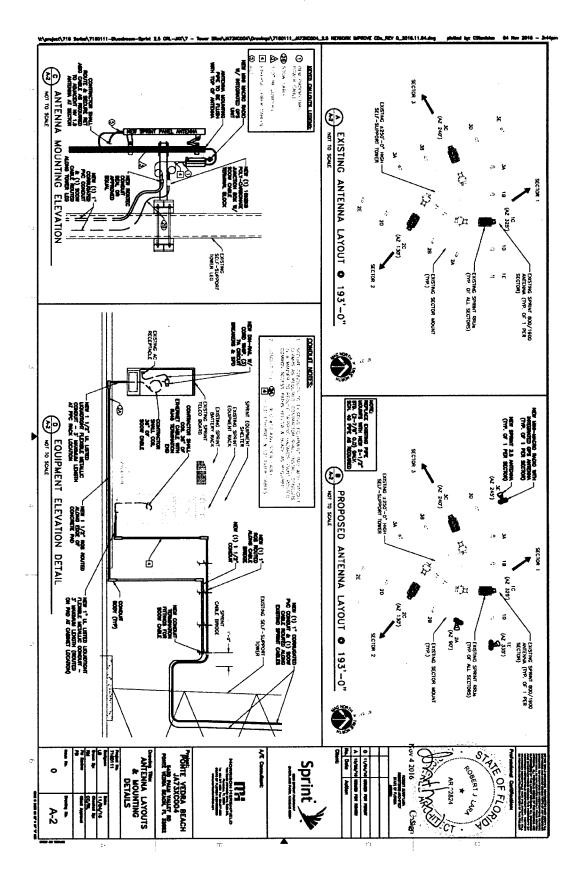
- 3 RFS APXVERR18-C Antennas
- 3 H+S TSZ 999 066/xxxM 1 1/4" lines
- 3 Ericsson 800 ESMR Filters
- 9 Ericsson ACU-A20-N RETS
- 3 Ericsson RRUS 11 @ 800 MHz
- 3 Ericsson RRUS 31 @ 1900 MHz
- 3 Alpha Wireless AW3286 Panel Antenna
- 3 Nokia FWHR TMA
- 1" Conduit with 3 Fiber and 3 Cat5e
- 1 SOOW power cable

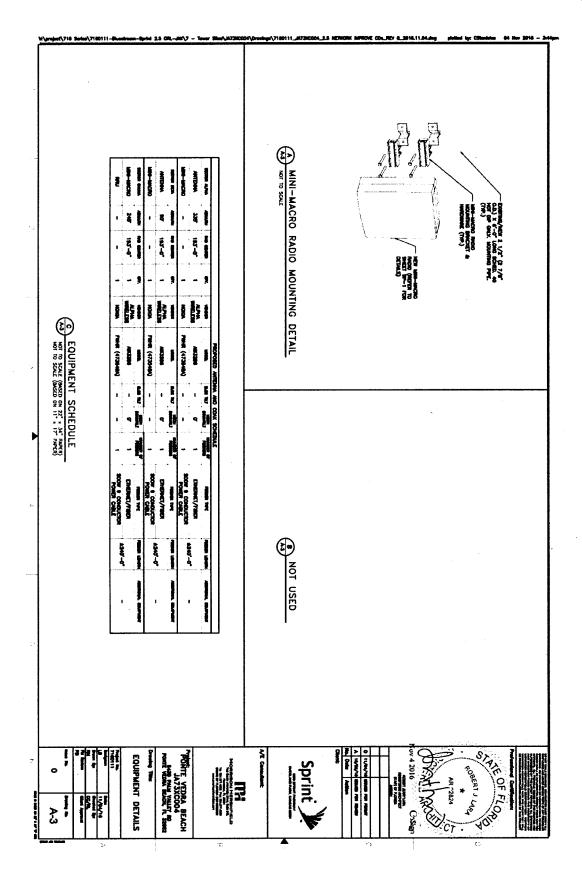


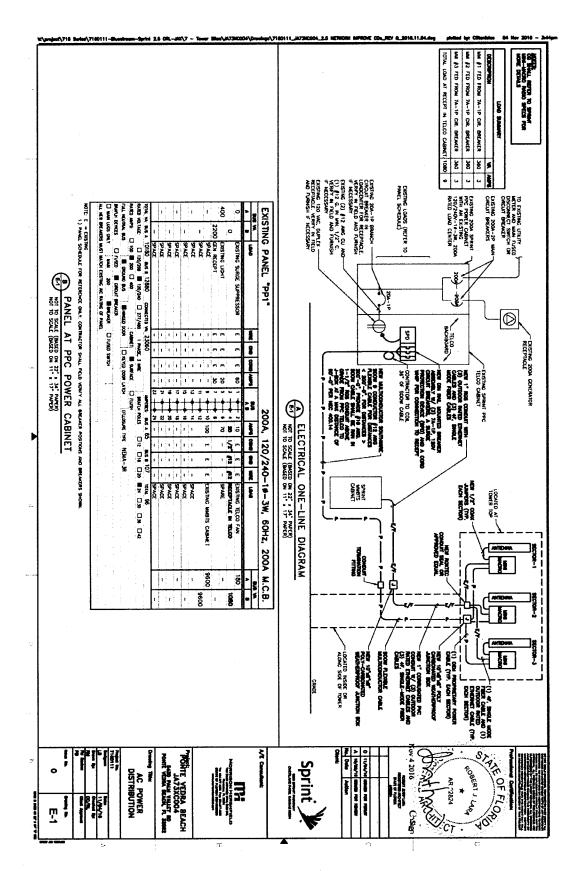


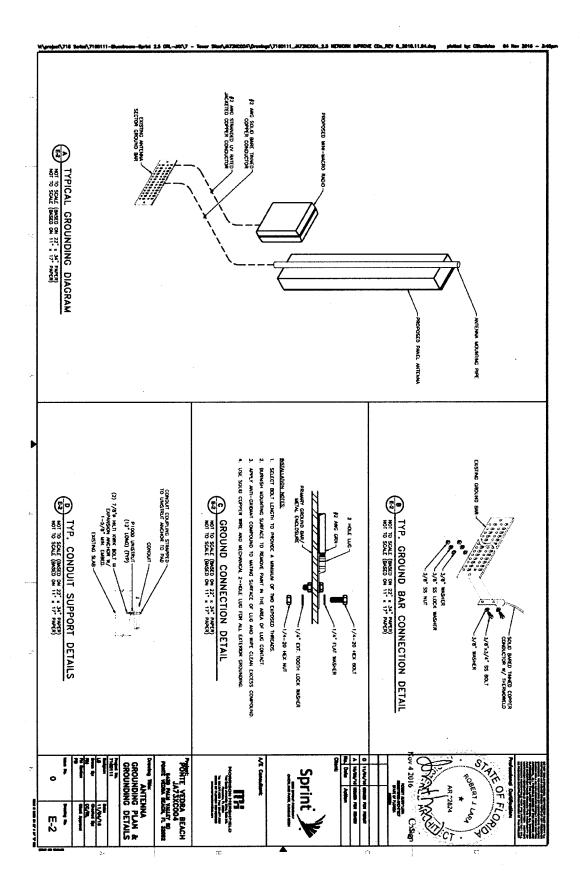














Ms. Kristen Beyer BlueStream Project Manager II (321) 262-4599

MORRISON HERSHFIELD

Morrison Hershfield Corporation 1455 Lincoln Parkway, Suite 500S Atlanta, GA 30346 (770) 379-8500

Date: October 27, 2016

Subject:

Structural Feasibility Report

Carrier: Carrier Site ID:

JA73XC004

Carrier Site Name:

Ponte Vedra Beach

Site Address:

5430 Palm Valley Rd., Ponte Veda Beach, St. Johns County, FL 32082

Site Coordinates:

Latitude 30° 11' 30.84", Longitude -81° 22' 58.08"

Tower Description:

250 ft - Self-Support Tower

Morrison Hershfield Project Number: NEV-539 / 7160111

Dear Ms. Fowler.

Morrison Hershfield Corporation has carried out a structural feasibility analysis of the above referenced structure for the existing and proposed antenna and equipment noted in Table 2. This feasibility analysis has been performed in accordance with the 2014 Florida Building Code, 5th Edition, based upon an ultimate 3-second gust wind speed of 142 mph converted to a nominal 3-second gust wind speed of 110 mph per section 1609.3.1 as required for use in the TIA-222-G Standard per Exception #5 of Section 1609.1.1. Exposure Category C with a maximum topographic factor, Kzt, of 1.0 and Risk Category II were used in this analysis.

Our analysis demonstrates that the existing tower IS in conformance (tower at 87.2%) with the requirements of the above noted standards under the effects of loading described. The capacity of the foundation CANNOT be determined. We recommend a foundation investigation to determine the as-built foundations be conducted so that an analysis of the foundation capacity can be completed.

We at Morrison Hershfield Corporation appreciate the opportunity of providing our continuing professional services to you and BlueStream. If you have any questions or need further assistance on this or any other projects please give us a call.

Sincerely, Morrison Hershfield Corporation

CáSign

6 12:17 PM

G. Lance Cooke, P.E. (FL License No. 68787) Senior Engineer

Certification of Authorization #8508

Moin sont Heisthhete

250 ft - Self-Support Tower - Structural Feasibility Report Project Number: NEV-539 / 7160111

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INTRODUCTION

This tower is a 250 ft guyed tower, and the original drawings are not available. The tower geometry and member sizes have been obtained from the previous structural analysis completed by SSOE Group, Project No. 014-01555-00, dated 10/23/2014 and are considered to be accurate.

This feasibility analysis was performed in accordance with the requirements of the 2014 Florida Building Code, 5th Edition, based upon an ultimate 3-second gust wind speed of 142 mph converted to a nominal 3- second gust wind speed of 110 mph per section 1609.3.1 as required for use in the TIA-222-G Standard per Exception # 5 of Section 1609.1.1. Exposure Category C, Topographic Category 1 and Risk Category II were used in this analysis. The design spectral response accelerations of Sps = 0.116 and Sp1 = 0.090 for Site Soil Class D were considered in this analysis.

Seismic design factors have been considered in this analysis. The seismic spectral response acceleration at short periods ($S_s = 0.109$) was determined to be less than 1.00; therefore as per ANSI/TIA-222-G Section 2.7.3 seismic effects have not been considered in this analysis.

The structural analysis was based on the following documentation:

Table 1 - Documentation

Document	Description	Source
Modification Drawings	Paul J. Ford and Co, Project No. 06406-0005, dated 09/05/2006	Client
Previous Structural Analysis	Paul J. Ford and Company, Project No. 06406-0005 Revision #2, dated 02/06/2007	Client
Previous Structural Analysis	SSOE Group, Project No. 014-01555-00, dated 10/23/2014	Client
As-Built Drawings	CLS Group, Site No. JA73XC004, dated 11/03/2015	Client
Proposed Loading	RFDS, Sprint Site No. JA73XC004, dated 10/20/2016	Sprint

1.0 ANALYSIS LOADING

The existing and proposed antennas, transmission lines, and other equipment considered in this analysis were provided by the client and are noted in Table

Table 2 - Antenna Loads

Elev. (ft)	QTY.	Antenna/Appurtenance Description	Carrier	QTY.	TX-Lines	Notes
		PROPOSED				
193.0 3		Alpha Wireless AW3286 Panel Antenna Sprint Nokia FWHR TMA		1	SOOW Power 1" Conduit	1
				3	Cat 5e Fiber Cable	
		EXISTING		· / ············		
256.0	4	RFS/Celwave BMR12A Omni			3/4" Conduit 1/2" 7/8"	
251.0	1	Decibel DB589 Omni		1 1		2
	1	Decibel DB220-A Omni	- suc	1 3		
0.46.0	4 Amphenol TTA TMA			•		
246.0 3 Side Arm Mount			,	-		
1		1 2' Dish				
240.0	1	Pipe Mount SJC				2
230.0	3	RFS/Celwave BMR10 Omni		3	4 5 /0"	
230.0	3 Side Arm Mount				1-5/8"	



(JA73XC004)

250 ft - Self-Support Tower - Structural Feasibility Report Project Number: NEV-539 / 7160111 October 27, 2016 Page 3

Elev. (ft)	QTY.	Antenna/Appurtenance Description	Carrier	QTY.	TX-Lines	Notes
		EXISTING [Cont'd]			
	10	Andrew TMBXX-6517-R2M Panel Antenna			1-5/8"	2
220.0	3	Nokia FRIG TMA	T-Mobile	12		
220.0	3	Nokia FXFB TMA] I-MODILE			2
	3	Sector Mount				
	3	RFS/Celwave APXVSPP18-C Panel Antenna			1/2"	
400.0	3	Ericsson RRU-11 RRU)i	3		2
193.0	3	Ericsson RRU-31 RRU	Sprint			-
	3	Sector Mount				
	6	Kathrein 840-10054 Panel Antenna		12	1-5/8"	2
168.0	2	2' Dish	Clearwire			
100.0	1	2.5' Dish	Clearwire			
	3	Sector Mount				
	1	2' Dish	ic A	1	7/8"	_
400.0	1	Pipe Mount	JEA			2
160.0	1	Andrew PAR6 Dish		1	EW63	2
	1	Pipe Mount	0.0			2
450.0	1	Amphenol BCD-87010 Omni	SJC	1	7/8"	
150.0	1	Side Arm Mount				2
00.0	1	Scala TY-900 Yagi	ICA.	1	1/2"	,
90.0	1	Side Arm Mount	JEA			2

Notes:

- Proposed equipment is in addition to the existing equipment at given elevation.
- 2) Existing loading is to remain on the tower.

ANALYSIS PROCEDURE

trixTower Version 7.0.7.0, a commercially available analysis software package, was used to create a threedimensional model of the tower and calculate member stresses for various loading cases. Selected output from the analysis is attached at the end of this report.

2.0 ASSUMPTIONS

The analysis provided by Morrison Hershfield is based on the theoretical capacity of the structure and is not a condition assessment of the tower. Morrison Hershfield has not performed an engineering inspection of the tower and the analysis was completed based on information supplied by the client. Morrison Hershfield has not made any independent determination of the accuracy of the information provided.

- Tower and structures were built in accordance with the manufacturer's specifications and the applicable ANSI/TIA/EIA standard.
- The tower and structures have been maintained in accordance with the manufacturer's specification.
- 3) The tower is assumed to be in good condition and capable of supporting its full design capacity.
- 4) The foundation was properly designed and constructed for the original design loads.
- 5) The configuration of antennas, transmission cables, mounts and other appurtenances are as specified in Table 2.



Site ID #: FL7041-A

Site Name: Ponte Vedra (JA73XC004)

250 ft - Self-Support Tower - Structural Feasibility Report Project Number: NEV-539 / 7160111 October 27, 2016 Page 4

- 6) All existing/proposed antennas and antenna mounts are assumed to be adequate for the existing/proposed loads. Analysis of these antennas and antenna mounts is considered to be outside of the scope of this analysis. Morrison Hershfield has not performed an analysis of the existing/proposed antennas or antenna mounts.
- 7) The existing loading was taken from previous structural analysis by SSOE Group, Project No. 014-01555-00, dated 10/23/2014 and As-built Drawings by CLS Group, Site No. JA73XC004, dated 11/03/2015, and is considered to be correct.
- The proposed loading was taken from RF Data Sheet, Sprint Site No. JA73XC004, dated 10/20/2016, and is considered to be correct.

If any assumptions are not valid or have been made in error, this analysis is invalid. Morrison Hershfield Corporation should be notified to determine the effect on the structural integrity of the tower.

3.0 SUMMARY OF RESULTS

The following tables summarize the location and utilized percentage of available capacity for each component of the tower. With consideration to the appropriate safety factors, 100% represents the full capacity of the component. Percentages below 100% indicate available capacity and conformance of the component. Percentages between 100% and 105% indicate an acceptable capacity. Percentages above 105% indicate an overstressed situation requiring structural modification to ensure conformance with the applicable codes and standards.

Based on our analysis results, the tower is within capacity to support the loads under the current loading scenario.

Table 3 - Tower Section Capacity

Section	Elevation	Component	Size	% Capacity	Pass
No.	ft	Type			Fail
T1	250 - 230	Leg	ROHN 3 STD	15.2	Pass
T2	230 - 210	Leg	ROHN 4 STD	29.2	Pass
T3	210 - 203.333	Leg	ROHN 5 STD	26.7	Pass
T4	203.333 - 196.667	Leg	ROHN 5 STD	30.2	Pass
T5	196.667 - 190	Lèg	ROHN 5 STD	32.7	Pass
T6	190 - 170	Leg	ROHN 5 STD	62.2	Pass
T7	170 - 150	Leg	ROHN 6 X-STR	40.1	Pass
T8	150 - 130	Leg	ROHN 8 X-STR	31.0	Pass
		-		36.7 (b)	
T9	130 - 110	Leg	ROHN 8 X-STR	37.9	Pass
T10	110 - 90	Leg	ROHN 10 X-STR	33.8	Pass
		-		33.9 (b)	
T11	90 - 70	Leg	ROHN 10 X-STR	38.5	Pass
T12	70 - 50	Leg	ROHN 12 EH	36.3	Pass
T13	50 - 30	Leg	ROHN 12 EH	40.6	Pass
T14	30 - 0	Leg	ROHN 12.75 EHS	37.9	Pass
T1	250 - 230	Diagonal	ROHN 2 STD	28.3	Pass
T2	230 - 210	Diagonal	ROHN 2 STD	61.1	Pass
T3	210 - 203.333	Diagonal	ROHN 2 STD	58.5	Pass
T4	203.333 - 196.667	Diagonal	ROHN 2 STD	61.3	Pass
T5	196.667 - 190	Diagonal	ROHN 2 STD	73.9	Pass
T6	190 - 170	Diagonal	ROHN 3 STD	37.1	Pass
T7	170 - 150	Diagonal	ROHN 3.5 STD	29.6	Pass
T8	150 - 130	Diagonal	ROHN 3.5 STD	35.5	Pass
T9	130 - 110	Diagonal	ROHN 3.5 STD	41.3	Pass
T10	110 - 90	Diagonal	ROHN 3.5 STD	51.0	Pass
T11	90 - 70	Diagonal	ROHN 3.5 STD	45.3	Pass
T12	70 - 50	Diagonal	ROHN 3.5 STD	51.4	Pass
T13	50 - 30	Diagonal	ROHN 3.5 X-STR	43.5	Pass
				48.0 (b)	
T14	30 - 0	Diagonal	ROHN 3.5 X-STR	48.6	Pass
T1	250 - 230	Horizontal	ROHN 1.5 STD	14.4	Pass
T2	230 - 210	Horizontal	ROHN 1.5 STD	36.0	Pass
T3	210 - 203.333	Horizontal	ROHN 2 STD	22.9	Pass
				23.5 (b)	_
T4	203.333 - 196.667	Horizontal	ROHN 2 STD	24.0	Pass
T5	196.667 - 190	Horizontal	ROHN 2 STD	27.5	Pass
T6	190 - 170	Horizontal	ROHN 2 X-STR	26.5	Pass



(JA73XC004)

250 ft - Self-Support Tower - Structural Feasibility Report Project Number: NEV-539 / 7160111

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Section No.	Elevation ft	Component Type	Size	% Capecity	Pass Fail
			P. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	28.9 (b)	
T7	170 - 150	Horizontal	ROHN 2.5 STD	26.8	Pass
T8	150 - 130	Horizontal	ROHN 3 STD	21.9	Pass
				28.3 (b)	_
T9	130 - 110	Horizontal	ROHN 3 STD	26.8	Pass
	448 88	44-4-1-4-1	5011114.075	30.1 (b)	_
T10	110 - 90	Horizontal	ROHN 3 STD	35.4	Pass
T11 T12	90 - 70 70 - 50	Horizontal Horizontal	ROHN 3 STD ROHN 3 X-STR	40.3 42.3	Pass Pass
T13	50 - 30	Horizontel	ROHN 3.5 STD	42.3 44.9	Pass
T14	30 - 0	Horizontal	ROHN 3.5 X-STR	44.0	Pass
	30 - 0	HOIZORE	KOHN 3.3 AGTK	47.5 (b)	Lass
T1	250 - 230	Top Girt	ROHN 1.5 STD	10.6	Pass
T5	196.667 - 190	Redund Horz 1 Bracing	L2x2x3/16	6.7	Pass
T11	90 - 70	Redund Horz 1 Bracing	ROHN 1.5 STD	32.1	Pass
T12	70 - 50	Redund Horz 1 Bracing	ROHN 1.5 STD	42.0	Pass
T13	50 - 30	Redund Horz 1 Bracing	ROHN 2 STD	28.8	Pass
T14	30 - 0	Redund Horz 1 Bracing	ROHN 1.5 STD	38.3	Pass
T14	30 - 0	Redund Horz 2 Bracing	ROHN 2 X-STR	49.3	Pass
T5	196.667 - 190	Redund Diag 1 Bracing	L2x2x3/16	7.5	Pass
T11	90 - 70	Redund Diag 1 Bracing	2L2x2x3/16x3/8	49.5	Pass
T12	70 - 50	Redund Diag 1 Bracing	2L2x2x3/16x3/8	55.4	Pass
T13	50 - 30	Redund Diag 1 Bracing	2L2x2x3/16x3/8	63.7	Pass
T14	30 - 0	Redund Diag 1 Bracing	2L2x2x3/16x3/8	70.5	Pass
T14	30 - 0	Redund Diag 2 Bracing	2L2x2x3/16x3/8	87.2	Pass
T11	90 - 70	Redund Hip 1 Bracing	ROHN 1.5 STD	0.3	Pess
T12	70 - 50	Redund Hip 1 Bracing	ROHN 1.5 STD	0.3	Pass
T13	50 - 30	Redund Hip 1 Bracing	ROHN 1.5 STD	0.4	Pass
T14	30 - 0	Redund Hip 1 Bracing	ROHN 1.5 STD	0.5	Pass
T14	30 - 0	Redund Hip 2 Bracing	ROHN 2 STD	0.5	Pass
T11	90 - 70	Redund Hip Diagonal 1 Bracing	ROHN 3 STD	0.5	Pass
T12	70 - 50	Redund Hip Diagonal 1 Bracing	ROHN 3 STD	0.5	Pass
T13	50 - 30 30 - 0	Redund Hip Diagonal 1 Bracing	ROHN 3 STD	0.5	Pass
T14	30 - 0 30 - 0	Redund Hip Diagonal 1 Bracing	ROHN 3 STD	0.7	Pass
T14	30 - 0	Redund Hip Diagonal 2 Bracing	ROHN 3 STD	0.6	Pass
T1 T2	250 - 230 230 - 210	Inner Bracing Inner Bracing	L2x2x1/8 L2x2x1/8	0.5 0.1	Pass Pass
T3	210 - 203.333	Inner Bracing Inner Bracing	L2x2x1/6 L2x2x1/8	0.1	Pass Pass
	203.333 - 196.667	Inner Bracing	L2x2x1/8 L2x2x1/8	0.1	Pass Pass
T5	196.667 - 190	Inner Bracing	L2x2x1/8	0.2	Pass
T6	190 - 170	inner Bracing	L2x2x1/8	0.2	Pass
17	170 - 150	Inner Bracing	L2 1/2x2 1/2x3/16	0.3	Pass
T8	150 - 130	Inner Bracing	L3x3x3/16	0.2	Pass
T9	130 - 110	Inner Bracing	L3 1/2x3 1/2x1/4	0.2	Pass
T10	110 - 90	Inner Bracing	L3 1/2x3 1/2x1/4	0.2	Pass
T11	90 - 70	Inner Bracing	ROHN 3 STD	0.2	Pass
T12	70 - 50	inner Bracing	ROHN 3 STD	0.3	Pass
T13	50 - 30	Inner Bracing	ROHN 3 STD	0.3	Pass
T14	30 - 0	Inner Bracing	ROHN 3 STD	0.3	Pass
		-		Summary	
			Leg (T6)	62.2	Pess
			Diagonal (T5)	73.9	Pass
			Horizontal (T14)	47.5	Pass
			Top Girt (T1)	10.6	Pass
			Redund Horz 1 Bracing (T12)	42.0	Pass
			Redund Horz 2 Bracing (T14)	49.3	Pass
			Redund Diag 1 Bracing (T14)	70.5	Pass
			Redund Diag 2 Bracing (T14)	87.2	Pass
			Redund Hip 1 Bracing (T14)	0.5	Pass
			Redund Hip 2 Bracing (T14)	0.5	Pass
			Redund Hip Diagonal 1 Bracing (T14)	0.7	Pess
			Redund Hip Diagonal 2 Bracing (T14)	. 0.6	Pass
			Inner Bracing (T1)	0.5	Pass
			Boit Checks	48.0	Pass



(JA73XC004)

Site ID #: FL7041-A

250 ft - Self-Support Tower - Structural Feasibility Report Project Number: NEV-539 / 7160111 October 27, 2016 Page 6

Capacity of Additional Components

Component	% Capacity	Pass/Fail	
Anchor Rods	31.1	Pass	

4.0 RECOMMENDATIONS

- All assumptions made in this analysis should be carefully reviewed. Morrison Hershfield should be contacted for any discrepancies so that a full assessment may be made to validate the results of this analysis.
- Morrison Hershfield strongly recommends that a foundation investigation to determine the as-built foundations be conducted so that the foundation can be properly analyzed prior to installing any additional equipment or make any other changes to the structure.

ATTACHMENTS: Tower Profile, Program Output, Coax Sketch and Additional Calculations.



Site ID #: FL7041-A

THE REMAINING PAGES OF THE 23 PAGE STRUCTURAL FEASIBILITY REPORT DATED OCTOBER 27, 2016, IS ON FILE AT THE ST. JOHNS COUNTY REAL ESTATE DIVISION, 500 SAN SEBASTIAN VIEW, ST. AUGUSTINE, FLORIDA 32084.